

Reliability-based design of roofs exposed to a snow load



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Introduction

Design alternatives

Reliability analysis

Concluding remarks



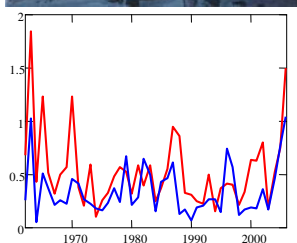
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1

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Introduction

- Collapses of roofs during the winter 2005/2006 – *is the reliability of roofs exposed to a snow load adequate?*
- Major *causes of failures* – extreme loads due to *snow*, insufficient *code provisions*, *errors* in design, execution and use
- Available *measurements* of snow loads newly evaluated and *standards revised* - the characteristic value as the 0.98 fractile of annual extremes and the partial factor 1.5
- The study focused on a critical analysis of present *design procedures in Eurocodes* using probabilistic methods



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2

Design alternatives

- Design of a *generic steel member* exposed to permanent action and snow load according to the load combination (6.10) in EN 1990
- *Alternatives*
 - Recommended *partial factors* (γ 's):
 $\gamma_{M0} = 1.0$; $\gamma_G = 1.35$; $\gamma_Q = 1.5$
 - Recommended *sensitivity factors* (α 's) - combination (6.10a,b):
 $\gamma_{M0} = 1.14$; $\gamma_G = 1.35(1.15)$; $\gamma_Q = 2.11(1.33)$
 - Partial factor dependent on the *load ratio* $\chi = s_{s,k} / (g_k + s_{s,k})$:
 $\gamma_Q = \gamma_{sd}(1 + \chi)$
 - *Reliability-based design* - partial factors derived from actual sensitivity factors obtained by FORM (data)

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3

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Probabilistic analysis

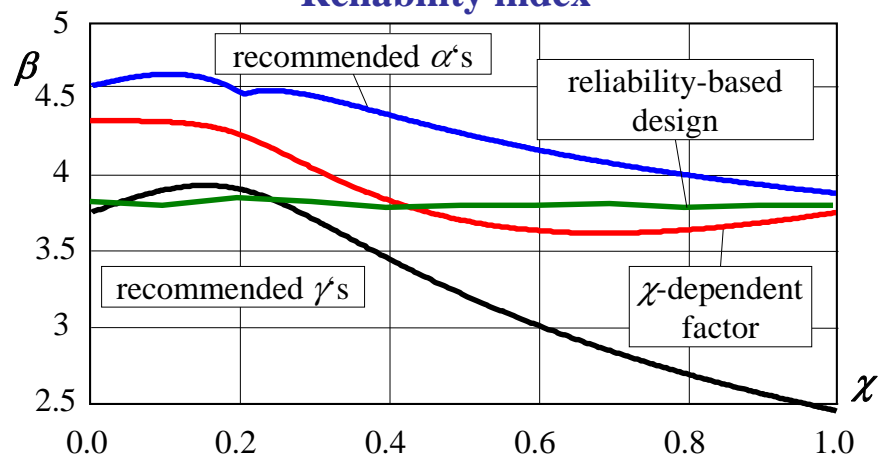
- *Limit state function*: $g(\mathbf{X}) = K_R R - K_E (G + \mu S_{50})$

Variable	Symb.	Distr.	Mean μ_X	CoV V_X
Resistance	R	Lognormal	$R_k \exp(2V_R)$	0.08
Permanent load	G	Normal	G_k	0.10
Shape coefficient	μ	Normal	0.8	0.15
Snow load on the ground, 50-year max.	S_{50}	Gumbel	s_k	0.22
Resistance uncert.	K_R	Lognormal	1.15	0.05
Load effect uncert.	K_E	Lognormal	1.0	0.10

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4

Reliability index



- Recommended γ 's - significant **variation** of the reliability index
- Partial factor γ_0 dependent on χ - **well-balanced** reliability
- Partial factors based on recommended α 's - **sufficient** reliability
- Reliability-based design – **target reliability** achieved

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5

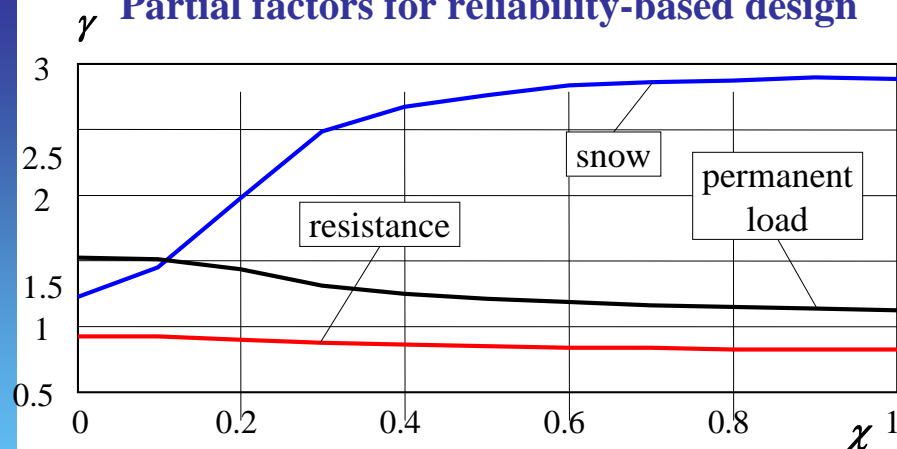
Conclusions

- The **constant partial factor** for snow load leads to a significantly **non-uniform reliability**.
- For steel roofs **reliability** of a structural member is **insufficient**.
- The partial factors should be determined on the basis of actual **sensitivity factors**.
- For steel roofs the **partial factor** for the snow load should be **greater than 1.5**.
- A more uniform reliability level may also be achieved using the **partial factor** for the snow load **dependent on the load ratio**.
- The results are dependent on the **assumed models**. Particularly models for the snow should be improved.

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7

Partial factors for reliability-based design



- The partial factor for **steel** close to the recommended value 1.0
- The partial factor for the **permanent load** in the range 1.1 - 1.5
- The partial factor of the **snow load** greater than 1.5
- Values of about **2.5 – 3.0** would lead to a sufficient reliability

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6

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Thank you for your attention.

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8