

Failures of Roofs under Snow Load: Causes and Reliability Analysis



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Introduction

Causes

Consequences

Reliability analysis

Concluding remarks

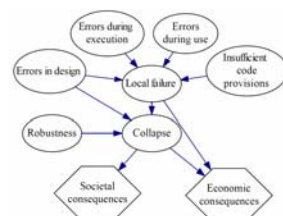


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Introduction

- **Number of collapses** of timber and steel roofs during the winter 2005/06 in Austria, Czech Republic, Germany and Poland
- Forensic assessments:
 - What were major **causes of failures**?
 - Were observed **snow** loads **exceptional**?
 - Is the **reliability** of structures designed according to standards **sufficient**?
- Presented overview of 249 investigations in the Czech Republic focused on **main causes, consequences** of failures and analysis of **code provisions**



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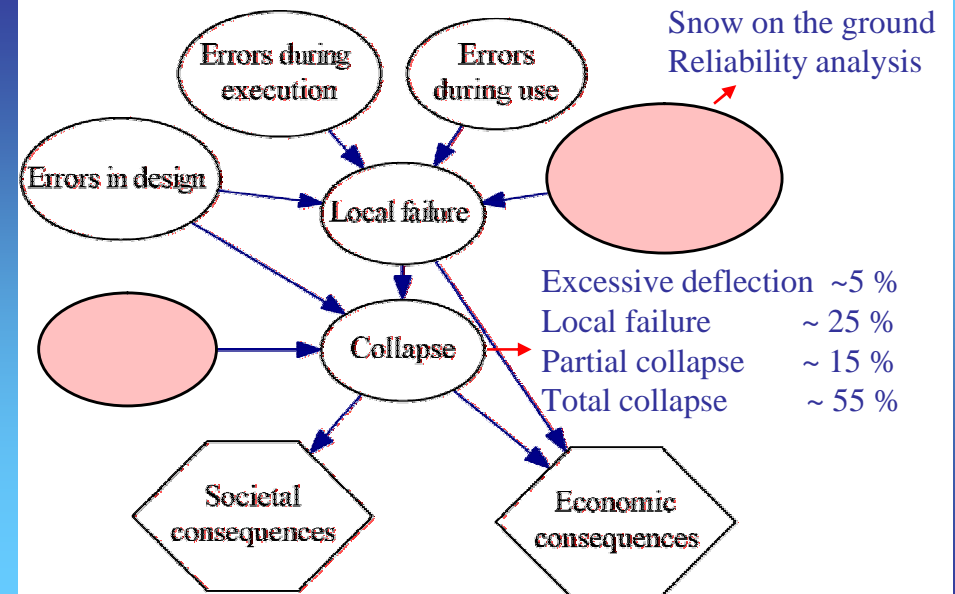
Causes of structural failures

- **Errors in design** (inconsistencies with code provisions, incorrect loading widths, numerical errors) – severe consequences
- **Errors during execution** (low-quality materials for timber structures, inappropriate details)
- **Errors during use** (incompetent interventions, installation of new facilities, insufficient maintenance) - most frequent errors
- **Insufficient code provisions** (low reliability of light-weight roofs, underestimation of load effects, influence of high-quality heat insulation, combination of snow and ice) - the most common cause
- In many cases **multiple causes** observed
- Errors in design and execution **not identified** due to inadequate **quality control**

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Consequences of structural failures



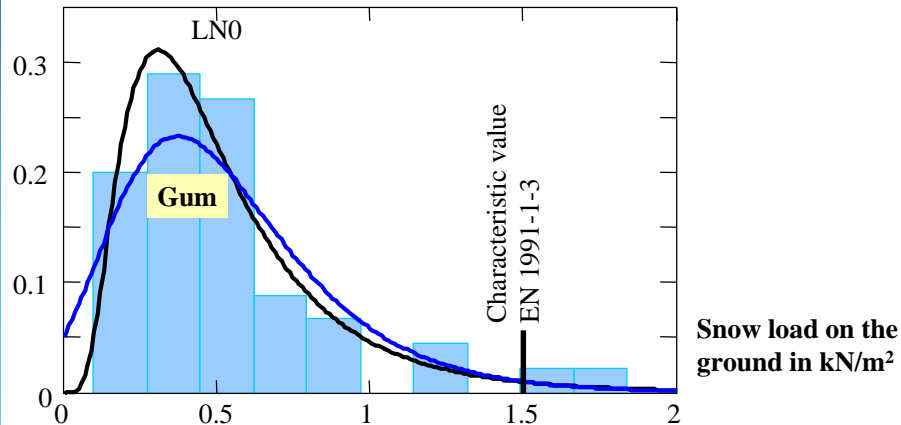
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Probabilistic model for the snow load on the ground

- *Annual maxima* – sample size 40-50 (no trends considered)
- *Coefficient of variation* about 0.7, *skewness* between 1.0 and 2.0

Relative frequency, probability density function



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Probabilistic reliability analysis

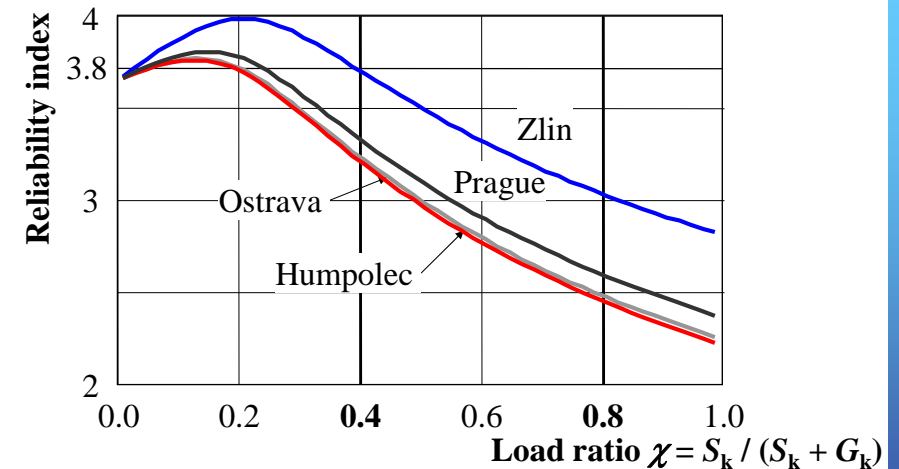
- Design of a *generic steel member* exposed to permanent and snow loads according to EN 1990 (2002)
- *Limit state function*: $g(\mathbf{X}) = K_R R - K_E (G + \mu S_{50})$

Variable	Symb.	Distr.	Partial factor	Mean	CoV
Resistance	R	LN	1	$1,17R_k$	0.08
Permanent load	G	N	1.35	g_k	0.10
Shape coefficient	μ	N	-	0.8	0.15
Snow on ground (50 years)	S_{50}	Gum	1.5	s_k	0.22
Resistance uncertainties	K_R	LN	-	1.15	0.05
Load effect uncertainties	K_E	LN	-	1.0	0.10

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Results of reliability analysis



- The recommended partial factors lead to a *significant variation* of the reliability index.
- For the load ratio 0.4 – 0.8 the reliability index decreases *below the target value* 3.8 recommended in EN 1990 (2002).

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Conclusions

- The main observed causes of structural damage may be subdivided into *errors* and *insufficient code provisions*.
- In some locations *snow loads* slightly exceeded characteristic values and may *hardly* be considered as *exceptional*.
- *Reliability* of roofs designed in accordance with the Eurocodes is *lower than* the *target level* recommended in EN 1990 (2002).
- Proper code provisions alone do not prevent collapses. *Systematic quality control* of design and execution needs to be applied.
- *Robustness* should be considered in design to *reduce* possible damage due to snow loads.

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